



System Prof	Grade EG/FG = Existing EG/FG = 26.0 (Guest)	-EG/FG = Existing
4" PVC solid pipe Sch. 40 **Inv. Main = ±25.3 Inv. Guest = ±24.0 **Inv. = 46' (Existing Tank 1' (Proposed Tan **Existing invert elevation to be verified at the time of construction.	$\frac{1}{22.4}$ $\frac{1}{22.4}$ $\frac{1}{22.4}$ $\frac{1}{22.4}$ $\frac{1}{22.4}$ $\frac{1}{22.4}$ $\frac{1}{22.2}$ $\frac{3'/7'}{\frac{1}{2}}$ $\frac{3'/7'}{\frac{1}$	Observation Pipe ***Top of Units = ± 22.4 ***EL = ± 21.1 Length = $37'-6''$ <u>6</u> Rows <u>6</u> Units per row <u>36</u> Total units required Groundwater NOT Found at Elev. = 12.7 H-20 HIGH CAPACITY LEACHING CHAMBER BED
stem Cross	Section	NOTE: Not to scale
OBSERVATION F NATIVE BACKFILI ***Top of Units 6" CLEAN SAND (SE 19'-6" ***Inv. = ±22.0	PIPES MATERIAL = ± 22.4 E NOTE 9) 6"	***Bottom of units El = ±21.1
NOIE: Leaching chamber rows are d within three vertical feet of bottom or H—20 loads and shall not be driver	to be spaced 6" apart. of excavation without the specific approval of n upon, even though H-20 leaching chambers	the design engineer. are specified.
a sewage disposal system and is State Environmental Code TITLE V wires, utilities or other underground ing these objects as necessary. apable of H-20 loading. ade so as to allow monitoring of id in relocating with metal m. high capacity biodiffusor or an 4% passing the No. 100 sieve. acility.	Design Cri Design Hydraulic Loading: 6 Bedrooms x 110 GPD/Bedroom = Septic tank capacity: Required: 660 GPD x 200% = 1320 Septic tanks provided = Two (2) 150 Leaching Capacity Provided: H-20 High Capacity Leaching Chamber 36 Leaching Chamber Units (26 exist 36 Units x 6.25 linear ft./unit x 4.7 1062 sq.ft. x 0.74 GPD/sq.ft. = 786	iteria 660 GPD Gal. minimum D0 Gal. (1 existing/1 proposed) er Bed ting + 10 additional) 2 sq.ft./linear ft. = 1062 sq.ft. 5 GPD (6 bedroom design) High capacity leaching chamber units are in bed configuration.
nd approve the installation and be provided to the owner and the	Proposed Septic S on Land in West Designed for: Madison Odenberg Street Address: 26 Chappaquidddick Assessor No.: 1–53 Lot Area: ±1.33 AC Designed By: Meegan Lancaster Checked By: R.G.S. Date: June 6, 2023 Revised: 11/21/2023 : guest house tank loco relocate pool	System Upgrade Tisbury, MASS. Avenue
	LAND SURVEYING	D 12 Cournoyer Road P.O. Box 421 West Tisbury, MA 02575 P 508-693-3774 F 508-629-0440 VLSE.net



Sheet Number	Sheet Name
Architectural	
A0.0	Cover Sheet
A0.1	Existing Site Plan
A0.2	Site Plan & Architectural Landscaping Plan
A1.0	Main House_Basement & Level 0.5 Floor Plans
A1.1	Main House_Level 1, 1.5. & 2 Floor Plans
A1.3	Main House_Basement & Level 0.5 RCPs
A2.0	Exterior Elevations_North & East
A2.1	Exterior Elevations_South & West
A2.2	Exterior 3D Views_Main House
A4.1	Interior Elevations_Kitchen&Dining
A4.2	Interior Elevations_Living Room
A4.4	Interior Elevations_Writing Room & Primary Hall
A4.6	Interior Elevations_Primary Bath
A5.0	Window & Exterior Door Schedules
A7.0	Garage/Guest House_Floor Plans & Elevations
A7.1	Garage Sections
Structural	
S0.0	General Notes
S1.0	Foundation Plan
S1.1	First Floor Framing Plan
S1.2	Second Floor Framing Plan
S1.3	Roof Framing Plan
S2.0	Structural Details

GENERAL NOTES

1. ALL WORK TO BE PERFORMED IN STRICT ACCORDANCE WITH THE REQUIREMENTS OF ALL GOVERNING CODES AND ORDINANCES, INCLUDING BUT NOT LIMITED TO: THE MASSACHUSETTS BUILDING CODE (9TH EDITION), MASSACHUSETTS FUEL, GAS, AND PLUMBING CODES, MASSACHUSETTS ELECTRICAL CODE, OSHA REGULATIONS, ORDINANCES OF THE TOWN OF WEST TISBURY, AND THE REQUIREMENTS OF THE WEST TISBURY FIRE DEPARTMENT.

2. THE CONTRACTOR SHALL VERIFY IN THE FIELD DIMENSIONS. THE CONTRACTOR SHALL VERIFY IN THE FIELD ALL CONDITIONS AFFECTING THE WORK. ANY CONDITIONS FOUND THAT ALTER OR OTHERWISE CHANGE THE REQUIREMENTS FOR THE WORK SHALL BE REPORTED IMMEDIATELY TO THE ARCHITECT.

3. THE CONTRACTOR SHALL COORDINATE THE WORK SHOWN ON THESE DRAWINGS WITH WORK OF ALL TRADES (MECHANICAL, PLUMBING, ELECTRICAL, ETC.) AND BRING ANY CONFLICT TO THE ATTENTION OF THE ARCHITECT.

LOT AREA IN TISBURY =	13,751 SF	
IOTAL LOT AREA =	57,931 SF	

Odenborg - Stupnitsky Residence

26 Chappaquiddick Ave, West Tisbury MA, 02568

GFA* RFA ** Main House Level 0.5 715 Level 1 1,365 (sf) 1.250 Level 1.5 845 Level 2 375 (sf) 330 screened porch (unconditioned) (sf) 235 3,300*** 2,340 total Garage/Guest House Garage (unconditioned) (sf) 345 (sf) **Guest House** (sf) Level 1 375 330 375*** 330 total 3,675 2,670 property total

* calculated from exterior walls

** calculated from interior of exterior walls

*** not including unconditioned spaces

ARCHITECT

TBD

Madison Odenborg & Gene Stupnitsky

CLIENT

Moskow Linn Architects 1693 Mass Ave, Cambridge, MA 02138

BUILDER

PRICING SET - NOT FOR CONSTRUCTION

A	Odenborg - Stupnitsky Residence	MOSKOW LINN ARCHITECTS, INC.	SCALE: As indicated	DATE	ISSUE
0.	26 Chappaquiddick Ave, West Tisbury MA 02568	88 BROAD STREET, BOSTON, MA 02110	DATE: 12.13.2023		
0	Cover Sheet	tel. 617.292.2000 fax. 617.426.4701 WWW.MOSKOWLINN.COM	DRAWN BY: MLA		















15'

25'



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 $1 \frac{\text{Main House - Basement RCP}}{3/16" = 1'-0"}$

	ISSUE		
	DATE		
	SCALE: 3/16" = 1'-0"	DATE: 12.13.2023	DRAWN BY: MLA
	MOSKOW LINN ARCHITECTS, INC.	88 BROAD STREET, BOSTON, MA 02110	tel. 617.292.2000 fax. 617.426.4701 WWW.MOSKOWLINN.COM
S1 S1 Image: Ext 2 Image: Ext 2 Image: Ext 1, Exterior Wall sconce Image: Ext 2, Exterior Downlight Image: Ext 2, Exterior Downlight Image: Physical Scince Image: Ext 2, Exterior Downlight Image: Physical Scince Image: Physical Scince	pnitsky Residence	ve, West Tisbury MA 02568	nent & Level 0.5 RCPs
P-2, 50° PENDANT FAN WITH LIGHT P-3, PENDANT AT STAIR P-4, PENDANT AT STAIR P-4, PENDANT AT GARAGE R-1, RECESSED CAN, 3° DIA. R-2, RECESSED CAN IN SHOWER, 3° DIA. R-3, RECESSED CAN IN SHOWER, 3° DIA. R-3, RECESSED CAN IN KITCHEN S-1, SURFACE MOUNT FIXTURE IN BASEMENT UC-1, UNDER-CABINET LED UL-1, UPLIGHTING IN LIVING ROOM P WS-1, WALL SCONCE P WS-3, WALL SCONCE AT BATHROOMS P WS-3, WALL SCONCE MOUNTED TO BEAM E EXHAUST FAN © SMOKE DETECTOR © CARBON MONOXIDE DETECTOR © HEAT DETECTOR IN GARAGE	V Odenborg - Stul	26 Chappaquiddick Av	Main House_Baserr

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 $1 \frac{\text{Main House - First Floor RCP}}{3/16" = 1'-0"}$

2 Main House - Second Floor RCP 3/16" = 1'-0"

LIGHTING F	IXTURE KEY
0-	EXT-1, EXTERIOR WALL SCONCE
0-	EXT-2, EXTERIOR DOWNLIGHT
\bigcirc	P-1, PENDANT IN KITCHEN
	P-2, 50" PENDANT FAN WITH LIGHT
\bigcirc	P-3, PENDANT AT STAIR
\bigcirc	P-4, PENDANT AT GARAGE
0	R-1, RECESSED CAN, 3" DIA.
0	R-2, RECESSED CAN IN SHOWER, 3" DIA.
0	R-3, RECESSED CAN IN KITCHEN
	S-1, SURFACE MOUNT FIXTURE IN BASEMENT
	UC-1, UNDER-CABINET LED
	UL-1, UPLIGHTING IN LIVING ROOM
	WS-1, WALL SCONCE
<u>_</u> <u>P</u>	WS-2, WALL SCONCE AT BATHROOMS
	WS-3, WALL SCONCE MOUNTED TO BEAM
E	EXHAUST FAN
S	SMOKE DETECTOR
\odot	CARBON MONOXIDE DETECTOR
hd	HEAT DETECTOR IN GARAGE

DATE

= 1'-0"

3/16"

SCALE:

ARCHITECTS, INC

MOSKOW LINN

12.13.2023

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A2.

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PER MASSACHUSETTS STRECH ENERGY CODE - PROJECT WILL COMPLY WITH OPTION 2 PERSCRIPTIVE PATH

TABLE REQUIREMENTS FROM INTERNATIONAL ENERGY CONSERVATION CODE - 2021 EDITION TABLE 402.1.3 INSULATION AND FENESTRATION REQUIREMENTS BY COMPONENT

	Climate Zone	Fenestration U-Factor	Skylight U-Factor	Glazed Fenestration SHGC	Ceiling R-Value	Wood Frame Wall R-Value	Mass Wall R-Value	Floor R-Value
Required	5	0.30	0.55	NR	60	30 or 20+5ci or 13+10ci or 0+20ci	13/17	30
Proposed	5	0.29	n/a	NR	60	33	n/a	43.9

2 Typical Mud Sill Detail 3/4" = 1'-0"

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Basement Wa R-Value	all Slab R-Value & Depth	Crawl Space Wall R-Value
15ci or 19 or 13+5ci	10ci, 4 ft	15ci or 19 or 13+5ci
15 + 11.6ci	11.6ci, 7.6ft	n/a
	 CEDAR SHINGLE SIDING 15/16" ZIP SHEATHING CLOSED CELL FOAM INS ON 2X6 WALLS, TYP. R6/I TOTAL WOOD FRAME W/ 	ULATION NCH. ALL
	PAINTED 5/8" GWB 11 7/8" AJS 20 WITH OPEI CELL FOAM INSULATION AT PERIMETER. R=43.9.	N _
	43.9. TYP. WATER TABLE	=
	(2) #4 - CONTINUOUS - 1" PIGID INSULATION ON	
	EXTERIOR OF FOUNDATI WALL.R=5	ION TION
	1" RIGID INSULATION.R=	5
	OPEN CELL FOAM INSULATION BETWEEN	
	9.7. TOTAL BASEMENT R- VALUE = 19.7	
	PAINTED 5/8" GWB	
	4" CONCRETE SLAB OVER 4" RIGID INSULATION, R=20 ON 8" LAYER OF 3/4" CRUSHED STONE. TOTAL SLAB R-VALUE = 20.0	
	6" DIA PERFORATED PVC PIPE	
	MIRAFFI FILTER FABRIC	OR
	2'-0"X1'-0" CONCRETE FOOTING OVER 8" OF COMPACTED LAYER OF 3/4" CRUSHED STONE	
	(2) #5 CONTINUOUS	

AΜ

4 Entry Hall_South 3/8" = 1'-0"

11 <u>Mudroom_South</u> 3/8" = 1'-0"

A2.1

AM <u> </u>

 $12 \frac{\text{Primary Hall}_South}{3/8" = 1'-0"}$

9 Writing room_4

ISSUE DATE = 1'-0" 12.13.2023 M 3/8" ВΥ: Ζ scale: DATE: DR **MOSKOW LINN ARCHITECTS, INC** DAD STREET, BOSTON, MA 02110 617.292.2000 fax. 617.426.4701 www.MOSKOWLINN.COM BRO, tel. 6 88 Stupnitsky Residence West Tisbury MA 02568 vations_Writing Room & Primary Hall ick Ave, ō I Č Odenborg •= σ 26 Chappa Interior Elev

A4.4

6 Enlarged Plan_Primary Bedroom 3/8" = 1'-0"

8 Primary Bedroom_East 3/8" = 1'-0"

ITEMS UPDATED: • CEILINGS NOW WHITE • CUSHIONS PULLED BACK, NOW CAP AT EDGE OF CUSHION (TO HOLD IT IN)

COMMENTS • BASEBOARD AT PANELING? • CANNOT CHANGE POST LOCATION (FOR STRUCTURAL REASONS) • POCKET DOOR INSTEAD OF CASED OPENING AT BEDROOM ENTRY?

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Level	Type Mark	Width	Height	Head Height	Count	Window Type	Tempere
Level -0.5'	4-C-t	2' - 9"	4' - 6"	7' - 0"	6	casement	yes
Level -0.5'	4-F-t	2' - 9"	4' - 6"	7' - 0"	2	fixed	yes
Level -0.5'	7-A-t-fr	2' - 9"	2' - 2"	7' - 0"	1	awning - frosted	yes
	1_	2' - 7 1/2"	1' - 6"	12' - 1"	9	fixed	
	2-ED-t	2' - 0"	7' - 0"		7	french door	VOS
	2-FD-L	2-9	1' 0"		7		yes
	2 E	5' 6"	1' 0"	7 - 0	5	fixed	
	3-F	<u> </u>	1-0	7 - 0 F' 0"	0		
	4-0	2 - 9	4 - 6	5-0	0	fixed	
	4-F	2 - 9	4 - 6	<valles></valles>	3	fixed	
	4-F-L	2 - 9	4 - 6	7 - 0	1	fixed	yes
	5-F	8 - 3	1 - 8	7 - 0	1	fixed	
	6-0	2'-9"	3' - 8"	5' - 0"	2	fixed	
_evel 1	6-C-t	2' - 9"	3' - 8"	7' - 0"	2	casement	yes
_evel 1	6-F	2' - 9"	3' - 8"	7' - 0"	3	fixed	
_evel 1	7-F-t	2' - 9"	2' - 2"	7' - 0"	3	fixed	yes
_evel 1	8-DD-t	2' - 9"	3' - 8"	7' - 0"	1	dutch door	yes
evel 1.5	2-FD-t	2' - 9"	7' - 0"	7' - 0"	1	french door	ves
evel 1.5	4-C	2' - 9"	4' - 6"	7' - 0"	18	casement	
evel 1.5	4-C-t	2' - 9"	4' - 6"	7' - 0"	4	casement	ves
_evel 1.5	4-F	2' - 9"	4' - 6"	7' - 0"	1	fixed	,
_evel 1.5	7-F-t	2' - 9"	2' - 2"	<varies></varies>	8	fixed	yes
	,				-		
_evel 2	2-FD-t	2' - 9"	7' - 0"	7' - 2"	4	<varies></varies>	<varies></varies>
_evel 2	4-C	2' - 9"	4' - 6"	7' - 6"	4	casement	
_evel 2	4-F	2' - 9"	4' - 6"	7' - 6"	2	<varies></varies>	
_evel 2	7-A-t	2' - 9"	2' - 2"	7' - 6"	3	awning	ves

2' - 9"

2' - 9"

Guest House Window & Exterior Dow Schedule Level Type Min/k Windb Height Count Windbw Type Tempered GH Level1 34 F C - 6 f 1 - 6 f	SCALE: 3/8" = 1'-0" DATE ISSUE SCALE: 12.13.2023 DATE ISSUE DRAWN BY: MLA DRAWN BY: MLA ISSUE
	MOSKOW LINN ARCHITECTS, INC. 88 BROAD STREET, BOSTON, MA 02110 tel. 617.292.2000 fax. 617.426.4701 www.MOSKOWLINN.COM
	Odenborg - Stupnitsky Residence 26 Chappaquiddick Ave, West Tisbury MA 02568 Window & Exterior Door Schedules
	A5.0

2-FD

GENERAL CONDITIONS

- 1. GENERAL CONTRACTOR TO CONSTRUCT FRAMING AS REPRESENTED IN THE STRUCTURAL DRAWINGS, COORDINATE WITH ARCHITECTURAL PLANS, AND FOLLOW STANDARD BUILDING PRACTICE. GENERAL CONTRACTOR RESPONSIBLE FOR IDENTIFYING AND CONFLICTS REPRESENTED IN THE FRAMING PLANS. ANY PROPOSED DEPARTURES FROM WHAT IS INDICATED MUST BE REVIEWED WITH THE ENGINEER PRIOR TO CONSTRUCTION. ALL UNAUTHORIZED CHANGES TO THE APPROVED DRAWINGS MUST BE REMOVED AND REPLACED AT THE CONTRACTOR'S EXPENSE.
- 2. THE CONTRACTOR SHALL CAREFULLY VERIFY ALL DIMENSIONS AND CONDITIONS SHOWN ON DRAWINGS PRIOR TO COMMENCEMENT OF THE WORK, AND SHALL NOTIFY THE ENGINEER IMMEDIATELY OF ANY DISCREPANCIES BETWEEN ENGINEERING AND ARCHITECTURAL DOCUMENTS.
- THE CONTRACTOR IS RESPONSIBLE FOR ALL MEANS AND METHODS OF TEMPORARY SHORING, BRACING, OR OTHERWISE PROTECTING ANY PORTION OF THE STRUCTURE, SITE AND UTILITIES FROM DAMAGE DURING CONSTRUCTION. THE ENGINEER IS SPECIFYING THE FINISHED CONDITION ONLY, WITHOUT ASSUMING KNOWLEDGE NOR RESPONSIBILITY FOR HOW THE CONTRACTOR WILL ACHIEVE THIS RESULT.
- 4. FOR RENOVATION WORK STRUCTURAL DRAWINGS PRODUCED WITH ASSUMPTIONS MADE REGARDING EXISTING CONDITIONS. IF CONTRACTOR FINDS EXISTING CONDITIONS NOT AS ASSUMED CONTACT ENGINEER IMMEDIATELY REVISIONS TO THE STRUCTURAL FRAMING MAY BE REQUIRED.
- 5. FOR EXACT LOCATIONS OF FLOOR AND ROOF OPENINGS, POSTS, ETC., SEE ARCHITECTURAL DRAWINGS.

FOUNDATIONS

- 1. WHERE FOUNDATIONS ARE EXISTING, DESIGN HAS BEEN COMPLETED ASSUMING FOUNDATIONS ARE SUITABLE TO SUPPORT PROPOSED RENOVATION. CONTRACTOR RESPONSIBLE FOR VERIFYING THAT THE EXISTING FOUNDATION CONFORMS TO BUILDING CODE REQUIREMENTS AND REPORT FOOTING CONDITIONS TO ENGINEER FOR VERIFICATION.
- 2. EXCAVATE TO LINES AND GRADES REQUIRED TO PROPERLY INSTALL THE FOUNDATIONS ON INORGANIC, UNDISTURBED SOIL OR CONTROLLED STRUCTURAL BACKFILL. ALL EXCAVATIONS SHALL BE DRY BEFORE PLACING ANY CONCRETE.
- 3. EXTERIOR FOOTINGS SHALL BE PLACED ON APPROVED SOIL AT A MINIMUM DEPTH OF 4 FEET, OR AS MODIFIED BY THE STRUCTURAL ENGINEER, BELOW THE LOWEST ADJACENT GROUND EXPOSED TO FREEZING. ANY ADJUSTMENT OF FOOTING ELEVATIONS DUE TO FIELD CONDITIONS MUST HAVE THE APPROVAL OF THE STRUCTURAL ENGINEER.
- 1. SOIL BEARING CAPACITY: FOOTINGS MUST BE PLACED ON SOIL WITH A MINIMUM BEARING CAPACITY OF 4000 POUNDS PER SQUARE FOOT.
- 5. BACKFILL BELOW FOOTINGS AND SLABS SHALL BE MADE WITH APPROVED GRANULAR MATERIALS PLACED IN 6" LAYERS. LAYERS SHALL BE COMPACTED TO 96% DENSITY AT OPTIMUM MOISTURE CONTENT. AS DEFINED BY ASTM D1557.
- 6. BACKFILLING AGAINST WALLS OR PIERS MAY ONLY BE DONE AFTER WALLS OR PIERS ARE BRACED TO PREVENT MOVEMENT. FOR WOOD FRAMED RESIDENTIAL CONSTRUCTION, NO BACKFILLING OF WALLS MAY TAKE PLACE UNTIL THE FIRST FLOOR DECK HAS BEEN FRAMED AND SHEATHED, UNLESS WRITTEN APPROVAL IS GIVEN BY THE ARCHITECT OR ENGINEER.
- 7. PROVIDE FOUNDATION DRAINAGE, WATERPROOFING/DAMP-PROOFING, AND FOUNDATION WALL INSULATION AS INDICATED ON THE ARCHITECTURAL DRAWINGS.

CONCRETE

- 1. ALL CONCRETE WORK SHALL BE PERFORMED IN CONFORMANCE WITH THE LATEST EDITION OF
- ACI-318, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE". CONCRETE SHALL ACHIEVE A MINIMUM 28 DAY DESIGN STRENGTH AS FOLLOWS: FOOTINGS, WALLS, INTERIOR SLABS-ON-GRADE, AND OTHER CONCRETE NOT OTHERWISE SPECIFIED - 3000 PSI.
- EXTERIOR SLABS EXPOSED TO WEATHER 4000 PSI. SLUMP AT THE POINT OF DISCHARGE FROM THE READY-MIX TRUCK SHALL BE 3-5".
- 4. REINFORCING STEEL: TYPICAL ASTM A615, GRADE 60. FIELD BENT ASTM A615, GRADE 40 WELDED WIRE FABRIC - ASTM A185.

STRUCTURAL STEEL

- 1. STRUCTURAL STEEL WORK SHALL CONFORM TO THE AMERICAN INSTITUTE OF STEEL
- CONSTRUCTION: "SPECIFICATION FOR STRUCTURAL STEEL FOR BUILDINGS", LATEST EDITION. 2. STEEL WIDE FLANGE BEAMS SHALL CONFORM TO ASTM A992, WITH A MINIMUM YIELD STRENGTH OF 50 KSI. PLATES, ANGLES, CHANNELS, AND MISC. FABRICATED HARDWARE SHALL CONFORM TO ASTM A36, WITH A MINIMUM YIELD STRENGTH OF 36 KSI. RECTANGULAR STEEL TUBING SHALL CONFORM TO ASTM A500, GRADE B, WITH A MINIMUM YIELD STRENGTH OF 46 KSI.
- 3. ALL STEEL TO STEEL FIELD CONNECTIONS SHALL BE MADE BY HIGH STRENGTH BOLTING WITH ASTM A325 BOLTS OR WELDING WITH E70 XX ELECTRODES. STEEL TO CONCRETE AND STEEL TO WOOD FIELD CONNECTIONS MAY BE MADE WITH ASTM A 307 BOLTS.
- 4. STEEL SHALL BE SHOP-PAINTED WITH A MODIFIED ALKYD PRIMER UNLESS OTHERWISE NOTED. 5. NO CUTTING OF OR OPENINGS THROUGH STEEL WILL BE PERMITTED WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.
- 6. CONTRACTOR TO SUBMIT SHOP DRAWING TO ARCHITECT AND ENGINEER FOR APPROVAL.

LATERAL FRAMING NOTES

- 1. THE STRUCTURAL DESIGN OF THIS RESIDENCE WAS PERFORMED IN COMPLIANCE WITH THE INTERNATIONAL RESIDENTIAL CODE FOR ONE AND TWO FAMILY DWELLINGS. THE PRESCRIPTIVE REQUIREMENTS OF THIS CODE DO NOT APPLY PER SECTIONS 301.1.3 ALTERNATIVE PROVISIONS AND 301.1.3 ENGINEERED DESIGN.
- 2. FRAMING COMPONENTS AND FASTENERS AS IDENTIFIED IN THESE DRAWINGS AND NOTES ADEQUATELY RESIST THE LATERAL LOAD REQUIREMENTS AS DEFINED BY THE INTERNATIONAL
- RESIDENTIAL CODE FOR ONE AND TWO FAMILY DWELLINGS. 3. ALL EXTERIOR WALLS TO FOLLOW SHEARWALL SHEATHING CRITERIA.
 - SHEARWALLS CONSTRUCTION: SHEATHING TO BE 1/2" APA RATED
- SHEATHING TO BE ATTACHED TO THE WALL STUDS WITH 8dNAILS @ 4" OC AROUND EDGES & 8" OC IN FIELDS. 4. HOLDDOWNS TO BE HDU5 BY SIMPSON AND SHALL BE ATTACHED TO A MIN OF 2-2x STUDS
- 5. THREADED ROD TO BE 5/8"ø AND EPOXY SHALL BE SIMPSON SET-XP
- 6. ALL PLYWOOD SEAMS IN A SHEARWALL SHALL BE BLOCKED WITH DIMENSIONAL LUMBER OF THE SAME SIZE AS THE WALL STUDS.
- 7. REFER TO PLANS AND SECTIONS FOR STUD SIZES, STUDS SHALL BE SPACED AT 16 INCHES ON CENTER UNLESS NOTED OTHERWISE ON PLAN.
- 8. CARE SHOULD BE TAKEN TO ADJUST NAIL GUN PRESSURE SO AS TO NOT OVER DRIVE NAILS INTO PLYWOOD. NAIL HEADS SHOULD BE FLUSH WITH PLYWOOD FACE. OVER DRIVING NAILS GREATLY
- REDUCES THE EFFECTIVENESS OF THE SHEARWALL. 9. FOR FRAMING SIZES REFER TO FRAMING PLANS.

ROUGH CARPENTRY

- IRC 1&2 OR IN THESE DRAWINGS.
- ENGINEER MAKES NO CLAIMS TOWARDS EXISTING CONDITIONS.
- #2 OR BETTER.
- THE CONTRACTOR.

- TREATED. FRAMING ELEMENTS BEFORE ORDERING. HANGERS UNLESS NOT
 - CONVENTIONAL 2x (KD) CONVENTIONAL MULTI I-JOISTS LVLS SINGLE PLY
 - LVLS 2 PLY LVLS 3 PLY

- OTHERWISE ON THE DRAWINGS.
- ONLY BE SPLICED OVER SUPPORTS. POINT
- SIZE.

1. ALL ROUGH CARPENTRY WORK SHALL BE EXECUTED IN CONFORMANCE WITH THE 9TH EDITION OF THE MASSACHUSETTS BUILDING CODE FOR ONE AND TWO FAMILY DWELLINGS (MBC 1&2) AND THE INTERNATIONAL RESIDENTIAL CODE FOR ONE AND TWO FAMILY DWELLINGS (IRC 1&2). REFER TO THE MBC 1&2 AND IRC 1&2 FOR FRAMING COMPONENTS NOT SPECIFIED IN PLANS AND SECTIONS. NOTIFY THE ENGINEER OF ANY COMPONENT NOT DEFINED IN EITHER THE MBC 1&2 AND

REFER TO THE IRC 1&2 FASTENER SCHEDULE FOR STRUCTURAL MEMBERS TABLE 602.3 FOR CONNECTION FASTENING NOT IDENTIFIED IN THESE PLANS OR DETAILS.

WHEN NOT OTHERWISE IDENTIFIED, ALL WOOD BEAMS, JOISTS, RAFTERS, HEADERS, STRINGERS, PLATES, AND SILLS SHALL BE SPRUCE PINE FIR #2 OR BETTER, WITH A MINIMUM Fb = 875 PSI (SINGLE USE) AND Fb = 1000 PSI (REPETITIVE USE), AND E SHALL BE 1,4000,000 PSI OR BETTER. 6. WOOD STUDS MAY BE EASTERN HEMLOCK, EASTERN SPRUCE, OR HEM-FIR, GRADED "STUD" GRADE,

7. LVL BEAMS, AS NOTED ON PLANS, SHALL HAVE A MINIMUM Fb = 3100 PSI, E = 2,000,000 PSI, AND Fv = 285 PSI. LVL BEAMS SHALL BE "VERSALAM" BY BOISE CASCADE. NO SUBSTITUTIONS WILL BE ACCEPTED, UNLESS THE ENGINEER SPECIFICALLY APPROVES ANOTHER PRODUCT SUBMITTED BY

8. LSL JOIST TO BE LAMINATED STRAND LUMBER BY TRUSJOIST OR EQUIVALENT. MINIMUM MATERIAL PROPERTIES TO BE Fb=2,300 PSI, Emin=1,550,000 PSI Fv=300 PSI. 9. WOOD I-JOISTS SHALL BE BY BOISE CASCADE. NO SUBSTITUTIONS WILL BE ACCEPTED, UNLESS THE ENGINEER SPECIFICALLY APPROVES ANOTHER PRODUCT SUBMITTED BY THE CONTRACTOR. MANUFACTURER'S RECOMMENDATIONS FOR BEARING, REINFORCING, CUTS, CANTILEVERS,

FASTENING, ETC., SHALL BE STRICTLY ADHERED TO. 10. ENGINEERED WOOD POSTS (VERSA COLUMNS), AS NOTED ON PLANS, SHALL BE VERSA-LAM 1.7 2650. 11. PLYWOOD WALL SHEATHING, ROOF SHEATHING, AND SUBFLOORING SHALL BE APA GRADE, TRADEMARKED C-D INTERIOR WITH EXTERIOR GLUE. SUBFLOORING SHALL BE 3/4" THICK TONGUE AND GROOVE, AND SHALL BE GLUED TO FLOOR JOISTS WITH AN APPROVED ADHESIVE PRIOR TO NAILING. ROOF SHEATHING SHALL BE 5/8" THICK AND WALL SHEATHING SHALL BE 1/2" THICK. 12. ALL WOOD HAVING DIRECT CONTACT WITH CONCRETE OR MASONRY, AND WHEREVER WOOD IS WITHIN 8" OF FINISHED GRADE OR PART OF OPEN DECK CONSTRUCTION, SHALL BE PRESSURE

13. ALL METAL CONNECTORS INCLUDING JOIST AND BEAM HANGERS AND COLUMN CAP AND BASES SHALL BE BY SIMPSON STRONG-TIE CORP. THE CONTRACTOR SHALL STRICTLY ADHERE TO MANUFACTURER'S FASTENING REQUIREMENTS. CONTRACTOR TO VERIFY ALL CONNECTOR SIZES TO

L ONDERNO.	
ED OTHERWISE	ARE AS FOLLOWS:
JOISTS	LUSxx
2x (KD) BEAMS	HHUSxx
. ,	IUSxx
	MIUxx
	HUxx
	HCUSYY

BEAMS TO COLUMNS (DIRECT CONTACT) 2-LCE4

14. UNLESS DETAILED OR SPECIFIED OTHERWISE ON THE PLANS, HEADERS AND BEAMS SHALL BE SUPPORTED BY AT LEAST ONE JACK STUD AND ONE KING STUD. 15. FOR WOOD JOIST SPANS UP TO 14 FEET, PROVIDE A SINGLE ROW OF FULL DEPTH BLOCKING

BETWEEN JOISTS AT MIDSPAN. FOR SPANS EXCEEDING 14 FEET, PROVIDE TWO ROWS OF FULL DEPTH BLOCKING BETWEEN JOISTS AT THIRD POINTS OF THE SPAN. 16. GABLE-END WALL STUDS IN CATHEDRAL, PARTIAL CATHEDRAL, OR HIGH CEILING SPACES SHALL SPAN UNINTERRUPTED FROM THE FLOOR PLATE TO THE UNDERSIDE OF THE ROOF RAFTERS. THEY SHOULD NOT BE INTERRUPTED BY ANY HORIZONTAL PLATES OR BEAMS, UNLESS NOTED

17. MEMBERS WITHIN BUILT-UP BEAMS, WHETHER MADE OF SAWN OR ENGINEERED LUMBER, SHALL

18. PROVIDE SIMPSON H1 OR H2.5 HURRICANE TIES BETWEEN EACH RAFTER BOTTOM AND ITS BEARING 19. CONTRACTOR SHALL CAREFULLY COORDINATE THE WORK OF ALL TRADES TO MINIMIZE THE NEED

FOR CUT, BORED OR NOTCHED IN FRAMING LUMBER. STRUCTURAL FLOOR MEMBERS SHALL NOT BE CUT, BORED OR NOTCHED IN EXCESS OF THE LIMITATIONS SPECIFIED IN THE BUILDING CODE WITHOUT WRITTEN APPROVAL FROM THE ENGINEER. 20. AT WOOD POSTS LANDING ON FLOOR DECK, PROVIDE SOLID VERTICAL WOOD BLOCKING WITHIN

DECK SANDWICH TO LINK UPPER POST WITH LOWER SUPPORT. BLOCKING TO MATCH UPPER POST 21. SET LVL BEAMS THAT FRAME FLUSH WITH DIMENSIONED LUMBER JOISTS 3/8" BELOW THE TOP OF JOISTS TO ALLOW FOR JOIST SHRINKAGE. WHERE BEARING WALLS OR POSTS LAND ON THESE

BEAMS. INFILL GAP WITH 3/8" PLYWOOD FOR SOLID BEARING. 22. BEAMS COMPRISED OF 3 LVLS OR MORE SHALL BE BOLTED TOGETHER WITH A MINIMUM OF 2-1/2". BOLTS AT 16" ON CENTER OR 3-1/4" • DIAMETER SELF TAPPING LAG SCREWS AT 16" ON CENTER,

ALTERNATING INSERTION SIDES. FOLLOW MANUF. SPECS. UNLESS NOTED OTHERWISE ON DRAWING. 23. IN ADDITION TO THE FLOOR JOIST SHOWN IN THE PLANS, CONTRACTOR SHALL INSTALL DOUBLE JOISTS UNDER ALL PARTITIONS WALLS RUNNING PARALLEL TO THE DIRECTION OF FRAMING. 24. MINIMUM BEAM BEARING TO BE 3 INCHES UNLESS NOTED OTHERWISE ON PLAN

LATERAL FRAMING LEGEND

- IDENTIFIES INTERIOR SHEARWALL SW

> IDENTIFIES HOLDOWNS (HD) INSTALLED ABOVE THE FRAMED LEVEL WITH ROD EXTENDING TO FRAMING BELOW. REFER TO SHEARWALL NOTES AND DETAILS

IDENTIFIES HOLDOWNS (HD) INSTALLED BELOW THE FRAMED LEVEL WITH ROD EXTENDING TO FRAMING ABOVE. REFER TO SHEARWALL NOTES AND DETAILS

IDENTIFIES STRAP HOLDOWN EXTENDING TO FRAMING ABOVE.

IDENTIFIES STRAP HOLDOWN EXTENDING TO FRAMING BELOW.

DESIGN LOADS PER MASSACHUSETTS STATE BUILDING CODE LIVE LOADS

GROUND SNOW LOAD:	40 PS
UNINHABITABLE ATTICS WITHOUT STORAGE:	: 10 PS
UNINHABITABLE ATTICS WITH LIMITED STOR	AGE: 20 PS
HABITABLE ATTICS AND SLEEPING AREAS:	30 PS
ALL OTHER AREAS:	40 PS

WIND LOADS

MASSACHUSETTS STATE BUILDING CODE : 100 MPH, EXPOSURE B

DEAD LOAD

WEIGHTS OF MATERIALS AND CONSTRUCTION PHOTOVOLTAIC PANEL SYSTEMS:

ABBREVIATIONS:

HDR - HEADER

D'L - ADDITIONAL KG - BLOCKING I - BEAM M - BOTTOM G - BEARING WN - BETWEEN V - BEARING WALL G - CEILING OL - COLUMN ONC - CONCRETE ONN - CONNECTION ONT - CONTINUOUS AG - DIAGONAL I - DOWN SSP - EQUAL SPACES - EACH FACE	JST - LSL - LVL - LW - MAX - MIN - MFR - NTS - OC - PL - PL - PT - REQ - SPEC - SW - TFN - TYP -	JOIST LAMINATED STRAND LUMBER LAMINATED VENEER LUMBER LONG WAY MAXIMUM MINIMUM MANUFACTURER NOT TO SCALE ON CENTER PLATE PRESSURE TREATED REQUIRED SPECIFICATION SHEAR WALL TOP FLANGE NAILER TYPICAL
)L - COLUMN	- OO	ON CENTER
G - CEILING	NTS -	NOT TO SCALE
JNN - CONNECTION	PT -	PRESSURE IREATED
NT - CONTINUOUS	REQ -	REQUIRED
AG - DIAGONAL	SPEC -	SPECIFICATION
I - DOWN	SW -	SHEAR WALL
SP - EQUAL SPACES	TFN -	TOP FLANGE NAILER
- EACH FACE	TYP -	TYPICAL
/ - EACH WAY	UON -	UNLESS OTHERWISE NOTED
N - FOUNDATION	VERT -	VERTICAL
I - FINISH	W/ -	WITH
G - FLANGE	WS -	STRUCTURAL GRADE WOOD SCREW 3/8"Ø,
G - FOOTING		UNLESS OTHERWISE NOTED.
RIZ -HORIZONTAL		SIMPSON SDS OR TIMBERLOK OR SIMILAR

5 PSF

MULTIPLE SPAN BEAM LEGEND

POST LEGEND

				I-JOIST ACCEP	TABLE ALTERNATIV	<u>ES</u>			
		1	_	ACCEDIADIC	ALTEDNATIVES		T BAIN SI	MOMENT AND SH	AP
OIST SERIES	DEPTH	FLANGE	TH	NORDIC	IPI	GP	FLx 106 [lb-in2]	Moment [ft-lhs]	Shear [lbs]
AJS 140	9-1/2"	2-1/2" FIR	TJI 210	NORDIC NI-40x	LPI 20 PLUS	GPI 40 / WI40	182	2450	1160
AJS 20	9-1/2"	2-1/2" FIR		NORDIC NI-60	LPI 32 PLUS		232	3395	1160
CI 6500s 1.8	9-1/2"	2-9/16" LVL		NORDIC NI-60	LPI 32 PLUS		220	3505	1575
AJS 25	9-1/2"	3-1/2" FIR		NORDIC NI-80	LPI 42 PLUS	1	322	5370	1160
AJS 140	11-7/8"	2-1/2" FIR	TJI 210	NORDIC NI-40x	LPI 20 PLUS	GPI 40 / WI40	310	3175	1490
AJ5 20	11-7/8"	2-1/2" FIR	TJI 360	NORDIC NI-60	LPI 32 PLUS	WI 60	394	4400	1490
CI 6500s 1.8	11-7/8"	2-5/16" LVL	TJI 360	NORDIC NI-60	LPI 32 PLUS	WI 60	365	4495	1675
BCI 60s 2.0	11-7/8"	2-5/16" LVL	TJI 560	NORDIC NI-80	LPI 42 PLUS	WI 80	450	6235	1675
AJS 25	11-7/8"	3-1/2" FIR	TJI 560	NORDIC NI-80	LPI 42 PLUS	WI 80	545	6960	1490
BCI 90s 2.0	11-7/8"	3-1/2" LVL			LPI 56	(675	9550	2150
AJS 140	14"	2-1/2" FIR	TJI 210	NORDIC NI-40x	LPI 20 PLUS	GPI 40 / WI40	457	3825	1790
AJS 20	14"	2-1/2" FIR	TJI 360	NORDIC NI-60	LPI 32 PLUS	WI 60	578	5295	1790
CI 6500s 1.8	14"	2-9/16" LVL	TJI 230	NORDIC NI-60	LPI 32 PLUS	WI 60	365	4495	1675
BCI 60s 2.0	14"	2-5/16" LVL	TJI 360	NORDIC NI-80	LPI 36	WI 80	450	6235	1675
AJS 25	14"	3-1/2" FIR	TJI 560	NORDIC NI-80	LPI 42 PLUS	WI 80	798	8380	1790
BCI 90s 2.0	14"	3-1/2" LVL	TJI 560	NORDIC NI-90	LPI 42 PLUS	WI 80	675	9550	2150
AJS 140	16"	2-1/2" FIR	TJI 210	NORDIC NI-60	LPI 20 PLUS	WI 60	623	4435	2065
AJS 20	16"	2-1/2" FIR	TJI 360	NORDIC NI-60	LPI 32 PLUS	WI 60	786	6140	2065
CI 6500s 1.8	16"	2-5/16" LVL	TJI 360	NORDIC NI-60	LPI 32 PLUS	WI 60	720	6085	2175
BCI 60s 2.0	16"	2-5/16" LVL	TJI 560	NORDIC NI-80	LPI 42 PLUS	WI 80	895	8520	2175
AJS 25	16"	3-1/2" FIR	TJI 560	NORDIC NI-80	LPI 42 PLUS	WI 80	1082	9720	2065
BC1 90s 2.0	16"	3-1/2" IVI	TII 560	NORDIC NI-90			1330	13050	2550

NOTE: FOR ALTERNATIVE NOT SHOWN REFER TO MINIMUM DESIGN PROPERTIES

or as allowed using BC Calc* sizing software.

-	

1. Square and rectangular holes are not permitted. larger hole.

AJS Joist Hole Location & Sizing

AJS" Joists are manufactured with 1/2" round perforated knockouts in the web at approximately 12" on center

Mini laros	mum spaci est hole (kr	ing = 2x gr nockouts e	eatest dim kempl)	ension of			1	D see table b	elow)	1			D	o NOT or notich Mange
	1	-]		0+						-6+	9)	Lod
(exc n be	ept knock; aring zone	outs) s.			A cl Pr	1%" round It anywhe rovide at h earance fr	I hole may re in the w east 3" of om other	l De Neb. Noles,	- 6"		6t • Do larg rou	not cut hol ler thân 1½ nd in canti	les)" levers,	aut in web area as specified
fre	om sup	oport,	listed	in tal	ole be	low, is	requ	ired fo	or all h	ioles g	greate	r than	11/2"	Select a table row based on
	3	4	5	6	6½	7	8	8%	9	10	11	12	13	joist depth and the actual joist span rounded up to the nearest table span. Scan across the row
	-	2	4	6	6	1	~				-			to the column headed by the appropriate round hole diamet
0"	2'-5"	2'-11"	3'-5"	3'-10"	4'-0"									or rectangular hole side. Use the longest side of a rectangular hole. The table value is the
0"	3'-8"	4'-5"	5'-1"	5'-10"	6'-0"									closest that the centerline of the hole may be to the centerline of
0*	4'-11"	5'-11"	6'-10"	7'-9"	8'-0°									 the nearest support. The entire web may be cut out.
	3	4	5	6	61/2	7	8	8%	9	10	Ħ	12	13	DO NOT cut the flanges. Holes apply to either single or multiple
	~	-	2	3	4	5	7	8	~	-	~	-	~	joists in repetitive member conditions.
0"	1'-5"	1'-10"	2'-3"	2'-8"	2'-11"	3'-1"	3'-6"	3'-11"						 For multiple holes, the amount of uncut web between holes must aqual at least twice the diameter.
5"	2'-1"	2'-9"	3'-5"	4'-0"	4'-4"	4'-8"	5'-4"	5'-11"						(or longest side) of the largest hole.
11"	2'-10"	3'-8"	4'-6"	5'-5"	5'-10"	6'-3"	7'-1"	7'-10"						 t½ⁿ round knockouts in the we may be removed by using a
5*	3'-6"	4'-7"	5'-8"	6'-9"	7'-3"	7'-10"	8'-11"	9'-10"						short piece of metal pipe and hammer.
5	3	4	5	6	6%	7	8	8%	9	10	11	12	13	 Holes may be positioned verti- cally in the web, provided they
	~		~	2	3	3	5	6	6	8	9	÷	×	 don't extend into either flange, This table was designed to
0"	1'-1"	1'-2"	1'-4"	1'-8"	1'-11"	2'-1"	2'-6"	2'-10"	2'-11"	3'-4"	3'-9"			apply to design conditions covered by uniform load PLF tables only shown elsewhere
0"	1'-1"	1'-4"	2'-0"	2'-7"	2'-11"	3'-2"	3'-10"	4'-4"	4'-5"	5'-0"	5'-7"			in this publication Use BC Calc [®] software to check other
0"	1'-1"	1'-10°	2'-8"	3'-5"	3'-10"	4'-3"	5'-1"	5'-9"	5'-11"	6'-8"	7'-6"			hole sizes or holes under other design conditions, including
0"	1'-3"	2'-4"	3'-4"	4'-4"	4'-10"	5'-4"	6'-4"	7'-3"	7'-4"	8'-5"	9'-5"			loads. It may be possible to exceed the limitations of this
0"	1'-7*	2'-9"	4*-0*	5'-2"	5'-10"	6'-5"	7'-8°	8'-8"	8'-10"	10'-1"	11'-3"			table by analyzing a specific application with the BC Calc ^{ar}
	3	4	5	6	6½	7	8	8%	9	10	11	12	13	software.
	-	-	8	+	-	2	3	5	5	6	8	9	10	
0*	1'-1"	1'-2"	1'-2"	1'-3"	1'-3"	1'-3"	1'-8"	2'-0"	2'-1"	2'-5+	2'-10"	3'-2"	3'-7"	
0"	$t^{\mu}\!\!=\!\!t^{\mu}$	1'-2"	1'-2"	1'-4"	1'-8"	1'-11'	2'-6"	3'-0"	3'-1"	3'-8"	4'-3"	4'-10"	5'-5"	
0"	1'-1"	1'-2"	1'-2"	1'-10"	2'-2"	2'-7"	3'-4"	4'-0"	4'-2"	4'-11"	5'-8"	6'-5"	7'-2"	
0*	1'-1"	1'-2"	1'-4"	2'-3"	2'-9"	3'-3"	4'-3"	5'-1"	5'-2"	6'-2°	7'-1"	8'-1"	9'-0"	
0"	1'-1"	1'-2"	1'-7"	2'-9"	3'-4"	3'-11"	5'-1"	6'-1"	6'-3"	7'-4"	8'-6"	9'-8"	10'-10"	

Large Rectangular Holes in AJS[®] Joists

Allowable Holes in Versa-Lam[®] LVL Beams

Cascade EWP Engineering.

ISSUE			
DATE			
SCALE: 1/4" = 1'-0"		DATE: 10/20/23	DRAWN BY:
	MOSKOW LINN ARCHITECTS, INC.	88 BROAD STREET, BOSTON, MA 02110	tel. 617.292.2000 fax. 617.426.4701 www.MOSKOWLINN.COM
V Webb			
	Odenborg - Stupnitsky Residence	26 Chappaquiddick Ave, West Tisbufy MA 02568	Main House_GENERAL NOTES
	S	0.	0

DATE ISSUE			
SCALE: 3/4" = 1'-0"		DATE: 10/20/23	DRAWN BY:
	MOSKOW LINN AKCHILECIS, INC.	88 BROAD STREET, BOSTON, MA 02110	tel. 617.292.2000 fax. 617.426.4701 WWW.MOSKOWLINN.COM
Vebb Structural	670 MAIN STREET READING MA 01867 781.779.1330		
	Odenborg - Stupnitsky Residence	26 Chappaquiddick Ave, West Tisbufy MA 02568	STRUCTURAL DETAILS
	SZ	2.	0

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Ŀ	1/4" = 1'-0"

